

Centre Scientifique et Technique du Bâtiment

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European Technical Assessment

ETA-19/0265 of 09/07/2019

English translation prepared by CSTB - Original version in French language

General Part

Nom commercial: Trade name:

Injection system AT-HP / AT-HP PLUS / S&P ResAC-19

Famille de produit : Product family:

Cheville à scellement pour fixation dans le béton fissuré et non fissuré: tiges filetées M8 à M24, fers à béton Ø8 à Ø25 Bonded injection type anchor for use in cracked and uncracked concrete: threaded rods size M8 to M24, rebar Ø8 to Ø25

Titulaire: Manufacturer: SIMPSON STRONG-TIE® 1 rue du camp ZAC des Quatre Chemins 85400 Sainte Gemme La Plaine France

Usine de fabrication: Manufacturing plant: SIMPSON STRONG-TIE®, Manufacturing Facilities

Cette évaluation contient: This assessment contains: 22 pages incluant 19 pages d'annexes qui font partie de cette évaluation

22 pages including 19 pages of annexes which form an integral part of this assessment

Base de l'ETE: DEE 330499-00-0601 Basis of ETA: EAD 330499-00-0601

Cette Evaluation remplace:

This Assessment replaces:

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Specific part

1 Technical description of the product

The injection system AT-HP / AT-HP PLUS / S&P ResAC-19 is a bonded anchor (injection type) consisting of a mortar cartridge with SIMPSON STRONG-TIE® resin AT-HP / AT-HP PLUS / S&P ResAC-19 and a steel element. The steel elements are threaded rods made of zinc coated steel, stainless steel, high corrosion resistant stainless steel (HCR), or rebar.

The steel element is placed into a drilled hole filled with injection mortar and is anchored via the bond between steel element, injection mortar and concrete. The steel element is intended to be used with embedment depth according Annex B5 - Table B4.

The illustration and the description of the product are given in Annexes A.

2 Specification of the intended use

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annexes B.

The provisions made in this European Technical Assessment are based on an assumed working life of the anchor of 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product

3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance under tension loads for threaded rod Acc. TR029	See Annex C1
Characteristic resistance under tension loads for rebars Acc. TR029	See Annex C2
Characteristic resistance under shear loads for threaded rods Acc. TR029	See Annex C3
Characteristic resistance under shear loads for rebars Acc. TR029	See Annex C4
Characteristic resistance under tension loads for threaded rods Acc. CEN/TS	See Annex C5
Characteristic resistance under tension loads for rebars Acc. CEN/TS	See Annex C6
Characteristic resistance under shear loads for threaded rods Acc. CEN/TS	See Annex C7
Characteristic resistance under shear loads for rebars Acc. CEN/TS	See Annex C8
Displacement for threaded rods and rebars	See Annex C9

3.2 Safety in case of fire (BWR 2)

Essential characteristic Performance	
Reaction to fire	Anchorages satisfy requirements for Class A1
Resistance to fire	No performance determined (NPD)

3.3 Hygiene, health and the environment (BWR 3)

Regarding dangerous substances contained in this European Technical Assessment, there may be requirements applicable to the products falling within its scope (e.g. transposed European legislation and national laws, regulations and administrative provisions). To meet the provisions of the Construction Products Directive, these requirements need also to be complied with, when and where they apply.

3.4 Safety in use (BWR 4)

For Basic Requirement Safety in use the same criteria are valid as for Basic Requirement Mechanical resistance and stability.

3.5 Protection against noise (BWR 5)

Not relevant.

3.6 Energy economy and heat retention (BWR 6)

Not relevant.

3.7 General aspects relating to fitness for use

Durability and Serviceability are only ensured if the specifications of intended use according to Annex B1 are kept.

4 Assessment and verification of constancy of performance (AVCP)

According to the Decision 96/582/EC of the European Commission¹, as amended, the system of assessment and verification of constancy of performance (see Annex V to Regulation (EU) No 305/2011) given in the following table apply.

Product	Intended use	Level or class	System
Bonded anchors for use in concrete	For fixing and/or supporting to concrete, structural elements (which contributes to the stability of the works) or heavy units	_	1

5 Technical details necessary for the implementation of the AVCP system

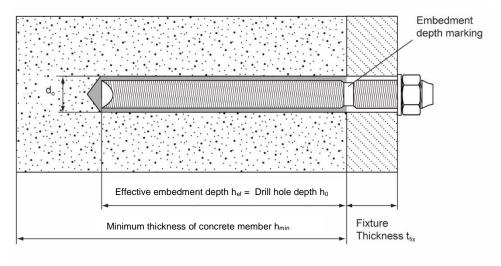
Technical details necessary for the implementation of the Assessment and verification of constancy of performance (AVCP) system are laid down in the control plan deposited at Centre Scientifique et Technique du Bâtiment.

The manufacturer shall, based on a contract, involve a notified body approved in the field of anchors for issuing the certificate of conformity CE based on the control plan.

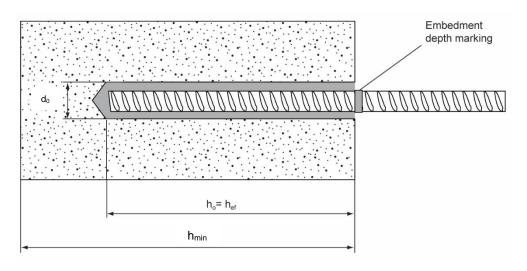
The original French version is signed by

Technical Director

Threaded rod M8, M10, M12, M16, M20, M24



Reinforcing bar Ø8, Ø10, Ø12, Ø14, Ø16, Ø20, Ø25 acc. to Annex 4

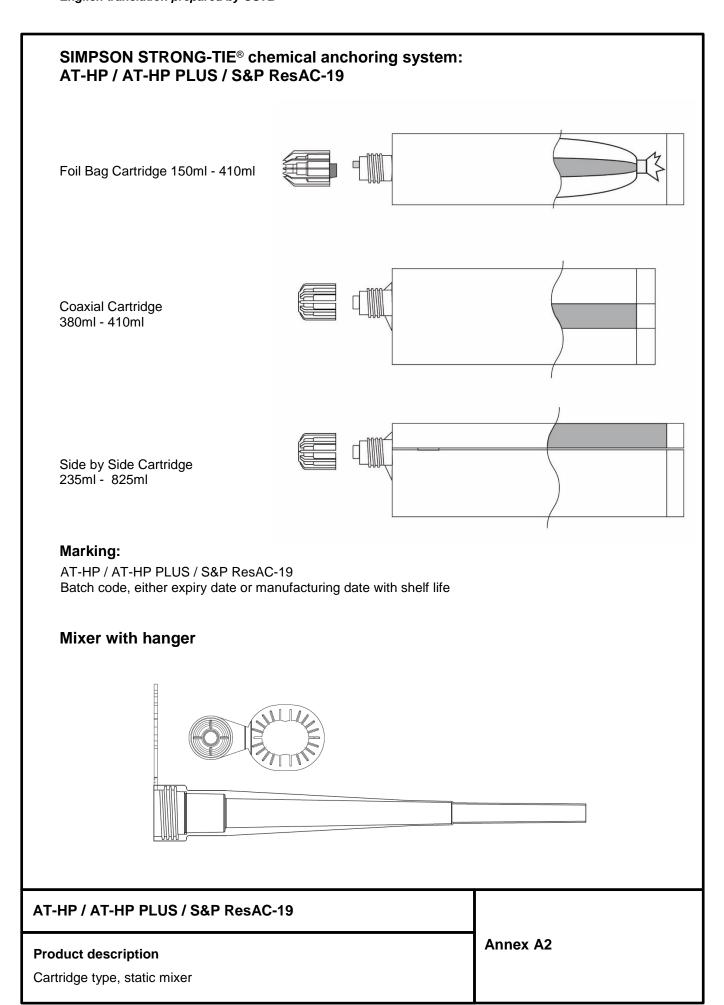


AT-HP	/ AT-HP	PLUS /	S&P	ResAC-19

Product description

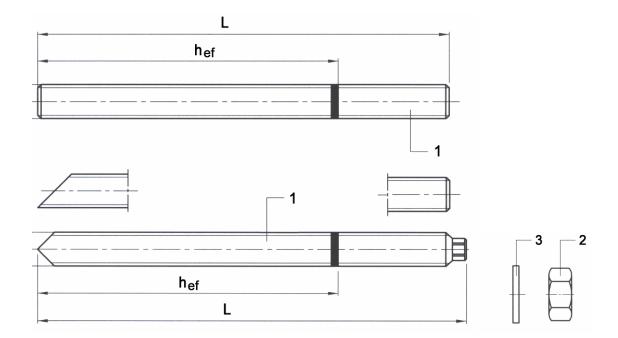
Installation condition

Annex A1



Anchor rod and rebar:

Threaded Steel Stud, Nut and Washer Sizes M8, M10, M12, M16, M20, M24.

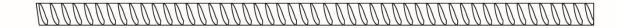


Commercial standard rod with:

- Materials, dimensions and mechanical properties (Table 1a)
- Inspection certificate 3.1 acc. to EN 10204:2004
- Marking of embedment depth

Rebar

Diameter Ø 8mm, Ø 10mm, Ø 12mm, Ø 14mm, Ø 16mm, Ø 20mm, Ø 25mm



AT-HP / AT-HP PLUS / S&P ResAC-19	
Product description Threaded rods and rebars	Annex A3

Table A1: Materials

Designation	Material				
Threaded rods made of zinc co	Threaded rods made of zinc coated steel				
Threaded rod M8 – M24	Strength class 5.8, 8.8, 10.9 EN ISO 898-1, $A_5 \ge 8\%$ ductile Electroplated zinc coating $\ge 5\mu m$ - EN ISO 4042, Hot dipped galvanized $\ge 45 \mu m$ EN ISO 10684				
Washer ISO 7089, ISO 7093, ISO 7094	Electroplated zinc coating ≥ 5µm EN ISO 4042; Hot dipped galvanized EN ISO 10684				
Nut EN ISO 4032 (DIN 934)	Strength class 8 - EN ISO 898-2 Electroplated zinc coating ≥ 5µm - EN ISO 4042, Hot dipped galvanized ≥ 45 µm EN ISO 10684				
Threaded rods made of stainle	ss steel				
Threaded rod M8 – M24	For ≤ M24: strength class 70 EN ISO 3506-1, A ₅ ≥ 8% ductile; Stainless steel 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362 EN 10088				
Washer ISO 7089, ISO 7093, ISO 7094	Stainless steel 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362 EN 10088				
Nut EN ISO 4032 (DIN 934)	Strength class 70 EN ISO 3506-2 Stainless steel 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362 EN 10088				
Threaded rods made of high co	orrosion resistant steel				
Threaded rod M8 – M24	For \leq M20: $R_m = 800 \text{ N/mm}^2$; $R_{p0,2} = 640 \text{N/mm}^2$, $A_5 \geq 8\%$ ductile For $>$ M20: $R_m = 700 \text{ N/mm}^2$; $R_{p0,2} = 400 \text{N/mm}^2$, $A_5 \geq 8\%$ ductile High corrosion resistant steel 1.4529, 1.4565 EN 10088				
Washer ISO 7089, ISO 7093, ISO 7094	High corrosion resistant steel 1.4529, 1.4565 EN 10088				
Nut EN ISO 4032 (DIN 934)	Strength class 70 EN ISO 3506-2 High corrosion resistant steel 1.4529, 1.4565 EN 10088				

Table A2: Properties of reinforcement bars (rebars)

Product form		Bars and de	-coiled rods
Class		В	С
Characteristic yield streng	th f _{yk} or f _{0,2k} (MPa)	400 to	600
Minimum value of $k = (f_t/f_y)k$		≥ 1,08	≥ 1,15 < 1,35
Characteristic strain at maximum force, ε _{uk} (%)		≥ 5,0	≥ 7,5
Bendability		Bend / Rebend test	
Maximum deviation from nominal mass ≤ 8 (individual bar) (%) > 8		±6 ±4	,
Bond: Minimum relative rib area, f _{R,min} (determination according to EN 15630) Nominal bar size (mm) 8 to 12 > 12		0,0 0,0	

Height of the rebar rib h_{rib} : The height of the rebar rib h_{rib} shall fulfil the following requirement: 0,05 * d \leq h_{rib} \leq 0,07 * d with: d = nominal diameter of the rebar

AT-HP / AT-HP PLUS / S&P ResAC-19	
Product description Threaded rods, Nuts, Washers, Rebars	Annex A4

Specifications of intended use

Anchorages subject to:

• Static and quasi-static loads

Base materials:

- Cracked concrete and uncracked concrete
- Reinforced or unreinforced normal weight concrete of strength classes C 20/25 at least to C50/60 at most according to EN 206: 2000-12.

Temperature Range:

- Installation: Base material ≥ 0°C / Bond material ≥ 20°C
- Ta: 0°C to +40°C (max. short term temperature +40°C and max. long term temperature +24°C)
- Tb: 0°C to +80°C (max. short term temperature +80°C and max. long term temperature +50°C)

Use conditions (Environmental conditions):

- Structures subject to dry internal conditions (zinc coated steel, stainless steel, high corrosion resistance steel).
- Structures subject to external atmospheric exposure (including industrial and marine environment) and to permanently damp internal conditions, if no particular aggressive conditions exists
 - (stainless steel, high corrosion resistance steel).
- Structures subject to external atmospheric exposure and to permanently damp internal condition, if other particular aggressive conditions exist (high corrosion resistance steel).

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used).

Design:

- The anchorages are designed in accordance with the EOTA Technical Report TR 029 "Design of bonded anchors" and CEN/TS 1992-4-5" Design of fastenings for use in concrete" under the responsibility of an engineer experienced in anchorages and concrete work
- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings.

Installation:

- Dry or wet concrete (category 1).
- Hole drilling by rotary hammer drill mode.
- Overhead installation is not permitted
- Installation in cracked concrete for threaded rods sizes M12 and M16 only
- Anchor installation carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site.

AT-HP / AT-HP PLUS / S&P ResAC-19	
Intended Use	Annex B1
Specifications	

Table B1: Drill hole cleaning method with Steel brush

Threaded rod and rebar	Size	Nominal drill bit diameter d _o (mm)	Steel Brush diameter d _b (mm)	Cleaning	methods
		8		Manual cleaning (MAC)	Compressed air cleaning (CAC)
	M8	10	12mm	Yes h _{ef} ≤ 80 mm	
Studs	M10	12	14mm	Yes h _{ef} ≤ 100mm	
	M12	14	16mm	Yes h _{ef} ≤ 120mm	Yes
	M16	18	20mm	Yes h _{ef} ≤ 160mm	
	M20	24	26mm	Yes h _{ef} ≤ 200mm	
	M24	28	30mm	Yes h _{ef} ≤ 240mm	
	Ø8	12	14mm	Yes h _{ef} ≤ 80 mm	
	Ø10	14	16mm	Yes h _{ef} ≤ 100mm	
Rebar	Ø12	16	18mm	Yes h _{ef} ≤ 120mm	
	Ø14	18	20mm	Yes h _{ef} ≤ 140mm	Yes
	Ø16	20	22mm	Yes h _{ef} ≤ 160mm	
	Ø20	25	28mm	Yes h _{ef} ≤ 200mm	
	Ø25	32	34mm	Yes h _{ef} ≤ 250mm	

Manual Cleaning (MAC):

SIMPSON STRONG-TIE® hand pump recommended for blowing out drill holes with diameters

d₀ ≤ 24 mm and drill hole depth h₀ ≤10d



Compressed air cleaning (CAC):

Recommended air nozzle with an opening of minimum 3,5 mm in diameter.



AT-HP / AT-HP PLUS / S&P ResAC-19	
Intended Use Cleaning tools	Annex B2

Table B2a: Installation parameters: drilling, hole cleaning and installation Instructions for use

Hole drilling Drill hole cleaning Just bet

Drill hole in the substrate to the required embedment depth using the appropriately sized carbide drill bit diameter d_{o} (mm).

Drill hole cleaning Just before setting an anchor, the drill hole must be free of dust and debris.

a) Manual air cleaning (MAC) for all hole diameters $d_0 \le 24$ mm and drill hole depth $h_0 \le 10$ d

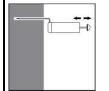
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The SIMPSON STRONG-TIE® manual pump shall be used for blowing out drill holes up to diameters $d_o \le 24$ mm and embedment depths up to $h_{ef} \le 10d$.

Blow out at least 4 times from the back of the drill hole, using an extension if needed.



X 4 Brush 4 times with the specified brush size (see Table B1) by inserting the SIMPSON STRONG-TIE® steel brush to the back of the hole (if needed with an extension) in a twisting motion and removing it.



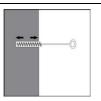
X 4 Blow out again with manual pump at least 4 times.

b) Compressed air cleaning (CAC) for all drill hole diameters do and all drill hole depths ho

6 Bar	

X 2

Blow 2 times from the back of the drill hole (if needed with a nozzle extension) over the whole length with oil-free compressed air (min. 6 bar at 6 m³/h).



Brush 2 times with the specified brush size (see Table B1) by inserting the SIMPSON STRONG-TIE® steel brush to the back of the drill hole (if needed with an extension) in a twisting motion and removing it.



X 2 Blow out again with compressed air at least 2 times.

AT-HP	/ AT-HP	PI US	/ S&P	ResAC-19
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Intended Use

Installation Instructions

Annex B3

Table B2b: Installation parameters: drilling, hole cleaning and installation

Instructions for use	
	Remove the threaded cap from the cartridge.
	Tightly attach the mixing nozzle. Do not modify the mixer in any way. Make sure the mixing element is inside the mixer. Use only the supplied mixer.
	Insert the cartridge into the SIMPSON STRONG-TIE® dispenser gun.
×	Discard the initial trigger pulls of adhesive. Depending on the size of the cartridge, an initial amount of adhesive mix must be discarded. Discard quantities are - 5cm for 150ml, 300ml & 400ml foil bag - cartridge - 10cm for all other cartridges
	Inject the adhesive starting at the back of the hole, slowly withdrawing the mixer with each trigger pull. Fill holes approximately 2/3 full, to ensure that the annular gap between the anchor and the concrete is filled with adhesive along the embedment depth. Setting control: Excess mortar flows out of the drill hole.
h _{ef}	Before use, verify that the threaded rod is dry and free of contaminants. Insert the threaded rod in a twisting motion to the required embedment depth during the open gel time t_{gel} has elapsed. The working time t_{gel} is given in Table B3.
t _{cure} T _{inst}	The anchor can be loaded after the required curing time t_{cure} (see Table B3). The applied torque shall not exceed the values T_{max} given in Table B4.

AT-HP / AT-HP PL	US / S&P ResAC-19	
Intended Use Installation Instruction	ns	Annex B3

Table B3: Minimum curing time

Mii	nim	num base material temperature C°	Gel time (working time) t _{gel} in dry/wet concrete	Cure time
0°	≤	T _{base material} < 5°C	25 min	90 min
5°C	≤	T _{base material} < 10°C	17 min	70 min
10°C	≤	T _{base material} < 20°C	12 min	65 min
20°C	≤	T _{base material} < 30°C	6 min	60 min
30°C	<u> </u>	T _{base material} ≤ 40°C	3 min	45 min

The temperature of the bond material must be $\geq 20^{\circ}$ C

AT-HP / AT-HP PLUS / S&P ResAC-19	
Intended Use Maximum working and minimum curing times	Annex B4

Table B4: Installation details for anchor rods

Anchor size	-	·	M8	M10	M12	M16	M20	M24
Diameter of anchor rod	d	[mm]	8	10	12	16	20	24
Effective embedment depth hef	min	[mm]	60	60	70	80	90	100
And drill hole depth h₀	max	[mm]	160	200	240	320	400	480
Nominal anchorage depth	h _{nom}	[mm]	80	90	110	125	170	210
Nominal diameter of drill hole	do	[mm]	10	12	14	18	24	28
Diameter of clearance hole in the fixture ¹⁾	df	[mm]	9	12	14	18	22	26
Maximum installation torque	T_{max}	[Nm]	10	20	30	60	90	140
Minimum thickness of concrete member	h _{min}	[mm]	h _{ef} + 30mm ≥ 100mm		ŀ	h _{ef} + 2d	0	
Minimum spacing	Smin	[mm]	40	50	60	80	100	120
Minimum edge distance	C _{min}	[mm]	40	50	60	80	100	120

1) For larger clearance hole see TR 029 section 1.1 or CEN/TS 1992-4-1 section 1.2.3

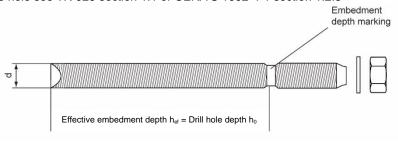
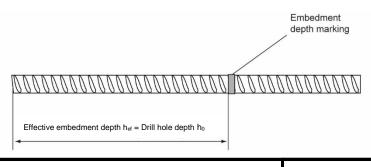


Table B5 - Installation details for rebars

Rebar diameter			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Diameter of rebar	d	[mm]	8	10	12	14	16	20	25
Effective embedment depth hef	min	[mm]	60	60	70	75	80	90	100
and drill hole depth h₀	max	[mm]	160	200	240	280	320	400	500
Nominal diameter of drill hole	d₀	[mm]	12	14	16	18	20	25	32
Minimum thickness of concrete member	h _{min}	[mm]	h _{ef} + 30mm ≥ 100mm			h _{ef} -	⊦ 2d₀		
Minimum spacing	Smin	[mm]	40	50	60	70	80	100	125
Minimum edge distance	C _{min}	[mm]	40	50	60	70	80	100	125



AT-HP / AT-HP PLUS / S&P ResAC-19	
Intended Use Installation parameters	Annex B5

AT-HP / AT-HP PLUS / S&P Rewith threaded rods	esAC-19		М8	M10	M12	M16	M20	M24	
Steel failure		•					•	•	
Characteristic resistance, class 5.8	$N_{Rk,s}$	[kN]	18	29	42	79	123	177	
Characteristic resistance, class 8.8	$N_{Rk,s}$	[kN]	29	46	67	126	196	282	
Partial safety factor	[-]			1	,5				
Characteristic resistance, class 10.9	$N_{\text{Rk,s}}$	[kN]	36	58	84	157	245	353	
Partial safety factor	γMs,N ¹⁾	[-]			1	.4			
Characteristic resistance, A4-70	$N_{Rk,s}$	[kN]	26	41	59	110	172	247	
Partial safety factor	γ _{Ms,N} 1)	[-]			1,	87			
Characteristic resistance, HCR	$N_{Rk,s}$	[kN]	29	46	67	126	196	247	
Partial safety factor	γMs,N ¹⁾	[-]			1,5			2,1	
Combined Pull-out and Concrete cone failure									
Diameter of threaded rod	d	[mm]	8	10	12	16	20	24	
Characteristic bond resistance in uncracked concrete C20/25									
Temperature range I ²⁾ : 40°C/24°C	$ au_{Rk,ucr}$	[N/mm²]	10,0	9,5	9,0	8,0	7,5	7,0	
Temperature range II ²⁾ : 80°C/50°C	τ _{Rk,ucr}	[N/mm²]	9,0	8,0	7,5	7,0	6,5	6,0	
		C30/37	1,12						
Increasing factor for TRk,p in uncracked concrete	ψc	C40/50	1,23						
in disclacked concrete		C50/60	1,30						
Characteristic bond resistance in crac	ked concre	te C20/25							
Temperature range I ²⁾ : 40°C/24°C	τ _{Rk,cr}	[N/mm²]	_ 5)	_ 5)	3,5	3,5	_5)	_ 5)	
Temperature range II ²⁾ : 80°C/50°C	$ au_{Rk,cr}$	[N/mm²]	_ 5)	_5)	3,0	3,0	_5)	_5)	
	-	C30/37	1,04						
Increasing factor for τ _{Rk,p} in cracked concrete	ψc	C40/50			1,	07			
in Gradited control		C50/60	1,09						
Splitting failure ²⁾									
	h / ł	n _{ef} ³⁾ ≥ 2,0	1,0	h _{ef}	h/h _{ef}				
Edge distance c _{cr,sp} [mm] for	2,0 > h /	h _{ef} ³⁾ > 1,3	4,6 h _{ef}	- 1,8 h	1,3				
	h /	$h / h_{ef}^{3)} \le 1,3$		2,26 h _{ef}			1,0·h _{ef} 2,26·h _{ef} c _{cr,sp}		
Spacing	S _{cr,sp} [mm]			2 Ccr,sp					
			1,5 ⁴⁾	1,5 ⁴⁾	1,5 ⁴⁾	1,5 ⁴⁾	1,5 ⁴⁾	1,5 ⁴⁾	

- 1) In absence of national regulations
- 2) Explanations, see Annex B1
- $^{3)}$ h = concrete member thickness, h_{ef} = effective embedment depth
- 4) The partial safety factor γ_2 = 1,0 is included
- 5) Not qualified in cracked concrete

AT-HP / AT-HP PLUS / S&P ResAC-19	
Design according to TR 029	Annex C1
Characteristic resistance under tension loads for threaded rods	

AT-HP / AT-HP PLUS / S&P Re with rebar	sAC-19		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25	
Steel failure rebar										
Characteristic resistance for rebar B500B acc. to DIN 488 ¹⁾	$N_{Rk,s}$	[kN]	28	43	62	85	111	173	270	
Partial safety factor for rebar B500B acc. to DIN 488 ²⁾	γMs,N ³⁾	[-]				1,4				
Combined Pull-out and Concrete cone failure										
Diameter of rebar	d	[mm]	8	10	12	14	16	20	25	
Characteristic bond resistance in uncr	acked cond	crete C20/2	5							
Temperature range I 4): 40°C/24°C	$ au_{Rk,ucr}$	[N/mm²]	7,0	7,5	7,0	7,0	6,5	6,5	6,0	
Temperature range II ⁴⁾ : 80°C/50°C	τ _{Rk,ucr}	[N/mm²]	6.5	6.5	6,0	6,0	6,0	5,5	5,5	
		C30/37 1,12								
Increasing factor for $\tau_{Rk,p}$	ψc	C40/50				1,23				
in uncracked concrete	C50/60		1,30							
Splitting failure										
	h /	′ h _{ef} ⁵⁾ ≥ 2,0	1,	0 h _{ef}	h/r 2,0	-				
Edge distance c _{cr,sp} [mm] for	2,0 > h /	h _{ef} ⁵⁾ > 1,3	4,6 h	_{ef} - 1,8 h	1,3	3				
	h / h _{ef} ⁵⁾ ≤ 1,3		2,2	2,26 h _{ef} 1,0·h _{ef} 2,2			2,26·h _{ef}	c _{cr,sp}		
Spacing	Scr,sp	[mm]	2 C _{cr,sp}							
Partial safety factor $\gamma_{Mp} = \gamma_{Mo}$	$c = \gamma Msp^{(3)}$	[-]	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	

- 1) The characteristic tension resistance N_{Rk,s} for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR 029, Equation (5.1).
- The partial safety factor $\gamma_{Ms,N}$ for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR 029, Equation (3.3a).
- 3) In absence of national regulations
- 4) Explanation see Annex B1
- $^{5)}~~h~\stackrel{\cdot}{=}$ concrete member thickness, h_{ef} = effective embedment depth
- $^{6)}$ $\,$ The partial safety factor γ_2 = 1,2 is included.

AT-HP / AT-HP PLUS / S&P ResAC-19	
Design according to TR 029	Annex C2
Characteristic resistance under tension loads for rebars	

AT-HP / AT-HP PLUS / S&P ResA with threaded rods	.C-19		M8	M10	M12	M16	M20	M24
Steel failure without lever arm								
Characteristic resistance, class 5.8	$V_{\text{Rk,s}}$	[kN]	9	15	21	39	61	88
Characteristic resistance, class 8.8	$V_{Rk,s}$	[kN]	15	23	34	63	98	141
Characteristic resistance, class 10.9	$V_{Rk,s}$	[kN]	18	29	42	79	123	156
Characteristic resistance, A4-70	$V_{Rk,s}$	[kN]	13	20	30	55	86	124
Characteristic resistance, HCR	$V_{\text{Rk,s}}$	[kN]	15	23	34	63	98	124
Steel failure with lever arm								
Characteristic resistance, class 5.8	M^0 Rk,s	[Nm]	19	37	66	167	326	561
Characteristic resistance, class 8.8	M ⁰ Rk,s	[Nm]	30	60	105	266	519	898
Characteristic resistance, class 10.9	M ⁰ Rk,s	[Nm]	38	75	131	333	649	893
Characteristic resistance, A4-70	$M^0_{Rk,s}$	[Nm]	26	53	92	233	454	786
Characteristic resistance, HCR	M ⁰ Rk,s	[Nm]	30	60	105	266	519	786
Partial safety factor steel failure								
grade 5.8 or 8.8	$\gamma_{Ms,V}^{1)}$	[-]			1,	25		
grade 10.9	γMs,V ¹⁾	[-]			1,	50		
A4-70	γ _{Ms,V} 1	[-]			1,	56		
HCR	γMs,V ¹⁾	[-]	1,25					
Concrete pryout failure								
Factor in equation (5.7) of Technical Report TR 029 for the design of bonded anchors	k	[-]	2					
Partial safety factor	γ _{Mcp} ¹⁾	[-]	1,5 ²⁾					
Concrete edge failure ³⁾				<u> </u>	<u> </u>	<u>-</u>		
Partial safety factor	γ _{Mc} ¹⁾	[-]	_	_	 1,{	5 ²⁾	_	_

¹⁾ In absence of national regulations

AT-HP / AT-HP PLUS / S&P ResAC-19	
Design according to TR 029	Annex C3
Characteristic resistance under shear loads for threaded rods	

²⁾ The partial safety factor γ_2 = 1.0 is included

 $^{^{3)}\,\,}$ Concrete edge failure see chapter 5.2.3.4 of Technical Report TR 029

		-							
AT-HP / AT-HP PLUS / S&P Res. with rebar	AC-19		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Steel failure without lever arm									
Characteristic shear resistance for rebar B500B acc. to DIN 4881)	$V_{Rk,s}$	[kN]	14	22	31	42	55	86	135
Partial safety factor for rebar B500B acc. to DIN 488 ²⁾	$\gamma_{\text{Ms,V}^3)}$	[-]				1,5			
Steel failure with lever arm									
Characteristic shear resistance for rebar B500B acc. to DIN 488 ⁴⁾	M^0 _{Rk,s}	[Nm]	33	65	112	178	265	518	1012
Partial safety factor for rebar B500B acc. to DIN 488 3)	γMs,V ³⁾	[-]				1,5			
Concrete pryout failure									
Factor in equation (5.7) of Technical Report TR 029 for the design of bonded anchors	k	[-]	2,0						
Partial safety factor	γMcp ³⁾	[-]	1,5 ⁵⁾						
Concrete edge failure ⁶⁾									
Partial safety factor	γMc ³⁾	[-]	•	•	•	1,5 ⁵⁾		•	•

- ¹⁾ The characteristic shear resistance $V_{Rk,s}$ for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR 029, Equation 5.5 or 5.6.
- The partial safety factor $\gamma_{\text{Ms,N}}$ for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR 029, Equation 3.3b or 3.3c.
- 3) In absence of national regulations
- The characteristic bending resistance M⁰_{Rk,s} for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR 029, Equation (5.6b).
- ⁵⁾ The partial safety factor $\gamma_2 = 1.0$ is included.
- 6) Concrete edge failure see chapter 5.2.3.4 of Technical Report TR 029

AT-HP / AT-HP PLUS / S&P ResAC-19	
Design according to TR 029	Annex C4
Design according to TK 029	
Characteristic resistance under shear loads for rebars	

AT-HP / AT-HP PLUS / S&P ResA(with threaded rods	3-19		М8	M10	M12	M16	M20	M24
Steel failure								
Characteristic resistance, class 5.8	N _{Rk,s}	[kN]	18	29	42	79	123	177
Characteristic resistance, class 8.8	$N_{Rk,s}$	[kN]	29	46	67	126	196	282
Partial safety factor	γMs,N ¹⁾	[-]			1,	5		
Characteristic resistance, class 10.9	$N_{Rk,s}$		36	58	84	157	245	353
Partial safety factor	γMs,N ¹⁾				1,	4		
Characteristic resistance "A4 70"	$N_{Rk,s}$	[kN]	26	41	59	110	172	247
Partial safety factor	γMs,N ¹⁾	[-]			1,8	37		
Characteristic resistance "HCR"	N _{Rk,s}	[kN]	29	46	67	126	196	247
Partial safety factor	γ _{Ms,N} 1)	[-]			1,5			2.1
Combined Pull-out and Concrete cone	failure							
Diameter of threaded rod	d	[mm]	8	10	12	16	20	24
Characteristic bond resistance in uncracke	ed concrete C20/2	25						
Temperature range I ²⁾ : 40°C/24°C	T _{Rk,ucr}	[N/mm²]	10,0	9.5	9.0	8.0	7.5	7.0
Temperature range II ²⁾ : 80°C/50°C	$ au_{Rk,ucr}$	[N/mm²]	9,0	8.0	7.5	7.0	6.5	6.0
		C30/37			1,	12		
Increasing factor for τ _{Rk,p}	Ψc	C40/50	1,23					
in uncracked concrete		C50/60			1,3	30		
Characteristic bond resistance in cracked	concrete C20/25							
Temperature range I ²⁾ : 40°C/24°C	$ au_{Rk,cr}$	[N/mm²]	_5)	_5)	3,5	3,5	_5)	_ 5)
Temperature range II ²⁾ : 80°C/50°C	$ au_{Rk,cr}$	[N/mm²]	_5)	_5)	3,0	3,0	_5)	_5)
Increasing factor for TRk		C30/37			1,0			
in cracked concrete	Ψc	C40/50			1,0			
	1-	C50/60			1,0			
Factor according to CEN/TS 1992-4-5 Section 6.2.2	K8 non cracked concre	ete [-]			10 7,	•		
Concrete cone failure	K8 cracked concrete	[-]			7,			
Factor according to CEN/TS 1992-4-5	k _{ucr}	[-]			10	.1		
Section 6.2.3	k _{cr}	[-]	7,2					
Characteristic edge distance	C _{cr} ,N	[-]			1,5	h _{ef}		
Characteristic spacing	S _{cr} ,N	[-]						
Splitting failure								
	h / h	ef ³⁾ ≥ 2,0	1,0	Nef	2,0			
Characteristic edge distance c _{cr,sp} [mm]	2,0 > h / h	n _{ef} ³⁾ > 1,3	4,6 h _{ef} -	4.0.5	1,3			
h / h _{ef} ³⁾ ≤		h _{ef} ³⁾ ≤ 1,3	2,26	h _{ef}		1,0·h _{ef}	2,26·h _{ef}	C _{cr,sp}
Characteristic spacing	Scr,sp	[mm]			2.0	cr,sp		

- 1) In absence of national regulations
- 2) Explanations, see Annex B1
- h = concrete member thickness, $h_{ef} = effective$ embedment depth
- The partial safety factor $\gamma_2 = 1.0$ is included
- 5) Not qualified in cracked concrete

AT-HP / AT-HP PLUS / S&P ResAC-19

Design according to CEN/TS 1992-4

Characteristic resistance under tension loads for threaded rods

Annex C5

AT-HP / AT-HP PLUS / S&P Res/ with rebar	AC-19		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Steel failure rebar									
Characteristic resistance for rebar B500B acc. to DIN 488 1)	N _{Rk,s}	[kN]	28	43	62	85	111	173	270
Partial safety factor for rebar B500B acc. to DIN 488 ²⁾	γMs,N ³⁾	[-]				1,4			
Combined Pull-out and Concrete con	e failure								
Diameter of rebar	d	[mm]	8	10	12	14	16	20	25
Characteristic bond resistance in uncrac	ked conc	crete C20/25	5						
Temperature range I ⁴⁾ : 40°C/24°C	$ au_{Rk,ucr}$	[N/mm²]	7,0	7,5	7,0	7,0	6,5	6,5	6,0
Temperature range II ⁴⁾ : 80°C/50°C	τ _{Rk,ucr}	[N/mm²]	6.5	6.5	6,0	6,0	6,0	5,5	5,5
	_	C30/37				1,12			
Increasing factor for $\tau_{Rk,p}$ in uncracked concrete	ψc	C40/50				1,23			
		C50/60				1,30			
Factor according to CEN/TS 1992-4-5 Section 6.2.2	uncracked o	concrete [-]				10,1			
Concrete cone failure									
Factor according to CEN/TS 1992-4-5 Section 6.2.2	ku	ıcr [-]				10,1			
Splitting failure									
$h / h_{ef}^{5)} \ge 2,0$ Edge distance $c_{cr,sp}$ [mm] for $2,0 > h / h_{ef}^{5)} > 1,3$		1,	0 h _{ef}	h/l 2,					
		h _{ef} ⁵⁾ > 1,3	4,6 h _{ef} - 1,8 h		1,3				
_	h/	h _{ef} ⁵⁾ ≤ 1,3	2,2	26 h _{ef}			1,0·h _{ef}	2,26·h _{ef}	C _{cr,sp}
Spacing	Scr,sp	[mm]				2 C _{cr,sp}			
Partial safety factor $\gamma_{Mp} = \gamma_{Mc} =$	= γ _{Msp} ³⁾	[-]	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶

- The characteristic tension resistance N_{Rk,s} for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR 029, Equation (5.1).
- The partial safety factor $\gamma_{Ms,N}$ for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR 029, Equation (3.3a).
- 3) In absence of national regulations
- 4) Explanation see Annex B1
- $^{5)}~~h=concrete$ member thickness, h_{ef} = effective embedment depth
- $^{6)}$ $\;$ The partial safety factor γ_2 = 1,2 is included.

AT-HP / AT-HP PLUS / S&P ResAC-19	
Design according to CEN/TS 1992-4	Annex C6
Characteristic resistance under tension loads for rebars	

AT-HP / AT-HP PLUS / S&P ResA with threaded rods	.C-19		M8	M10	M12	M16	M20	M24
Steel failure without lever arm								
Characteristic resistance, class 5.8	$V_{\text{Rk,s}}$	[kN]	9	15	21	39	61	88
Characteristic resistance, class 8.8	$V_{Rk,s}$	[kN]	15	23	34	63	98	141
Characteristic resistance, class 10.9	$V_{\text{Rk,s}}$	[kN]	18	29	42	79	123	156
Characteristic resistance, A4-70	$V_{Rk,s}$	[kN]	13	20	30	55	86	124
Characteristic resistance, HCR	$V_{Rk,s}$	[kN]	15	23	34	63	98	124
Steel failure with lever arm								
Characteristic resistance, class 5.8	$M^0_{Rk,s}$	[Nm]	19	37	66	167	326	561
Characteristic resistance, class 8.8	M ⁰ Rk,s	[Nm]	30	60	105	266	519	898
Characteristic resistance, class 10.9	M^0 Rk,s	[Nm]	38	75	131	333	649	893
Characteristic resistance, A4-70	M^0 Rk,s	[Nm]	26	53	92	233	454	625
Characteristic resistance, HCR	M^0 Rk,s	[Nm]	30	60	105	266	519	786
Partial safety factor steel failure								
grade 5.8 or 8.8	$\gamma_{Ms,V}^{1)}$	[-]			1,	25		
grade 10.9	γMs,V ¹⁾	[-]			1,	50		
A4-70	γMs,V ¹⁾	[-]			1,	56		
HCR	$\gamma_{Ms,V}$ 1)	[-]			1,25			1,75
Concrete pryout failure								
Factor according to CEN/TS 1992-4-5 Section 4.3.3	k ₃	[-]	2					
Partial safety factor	γMcp ¹⁾	[-]	1,5 ²⁾					
Concrete edge failure								
Partial safety factor	γMc ¹⁾	[-]			1,	5 ²⁾		

¹⁾ In absence of national regulations

AT-HP / AT-HP P	LUS / S&P ResAC-19

Design according to CEN/TS 1992-4

Characteristic resistance under shear loads for threaded rods

Annex C7

 $^{^{2)}}$ $\,$ The partial safety factor $\gamma_{2}\text{=}$ 1.0 is included

AT-HP / AT-HP PLUS / S&P Resolution with rebar	AC-19		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Steel failure without lever arm									
Characteristic shear resistance for rebar B500B acc. to DIN 4881)	$V_{Rk,s}$	[kN]	14	22	31	42	55	86	135
Partial safety factor for rebar B500B acc. to DIN 488 ²⁾	γMs,V ³⁾	[-]				1,5			
Steel failure with lever arm									
Characteristic shear resistance for rebar B500B acc. to DIN 4881)	$M^0_{Rk,s}$	[Nm]	33	65	112	178	265	518	1012
Partial safety factor for rebar B500B acc. to DIN 488 2)	γ _{Ms,V} ³⁾	[-]	1,5						
Concrete pryout failure									
Factor according to CEN/TS 1992-4-5 Section 6.3.3	k 3	[-]				2			
Partial safety factor	γ _{Mcp} ³⁾	[-]	1,5 ⁵⁾						
Concrete edge failure									
Partial safety factor	γMc ³⁾	[-]				1,5 ⁵⁾			

- ¹⁾ The characteristic shear resistance $V_{Rk,s}$ for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR 029, Equation 5.5 or 5.6.
- The partial safety factor $\gamma_{Ms,N}$ for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR 029, Equation 3.3b or 3.3c.
- 3) In absence of national regulations
- The characteristic bending resistance Mo_{Rk,s} for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR 029, Equation (5.6b).
- ⁵⁾ The partial safety factor γ_2 = 1,0 is included.

AT-HP / AT-HP PLUS / S&P ResAC-19	
Design according to CEN/TS 1992-4	Annex C8
Characteristic resistance under shear loads for rebars	

Displacement under tension load 1)

AT-HP / AT-HP PLUS / S&P ResAC-19 with threaded rods			M8	M10	M12	M16	M20	M24	
Uncracked concrete temperature range I: 40°C / 24°C									
Displacement	δηο	$[mm/(N/mm^2)]$	0,03	0,03	0,04	0,05	0,06	0,07	
Displacement	δ _{N∞}	[mm/(N/mm²)]	0,07	0,09	0,10	0,13	0,17	0,20	
Uncracked concrete te	ge II: 80°C / 50°C	C							
Displacement	δ_{N0}	$[mm/(N/mm^2)]$	0,04	0,04	0,05	0,07	0,08	0,10	
Displacement	δn∞	[mm/(N/mm²)]	0,10	0,13	0,15	0,19	0,23	0,28	
Cracked concrete temperature range I: 40°C / 24°C									
Displacement	δ_{N0}	[mm/(N/mm²)]	-	-	0,12	0,09	-	-	
Displacement	δ _{N∞}	[mm/(N/mm²)]	-	-	0,64	0,55	-	-	
Cracked concrete temperature range II: 80°C / 50°C									
Displacement	δηο	[mm/(N/mm²)]	-	-	0,17	0,13	-	-	
Displacement	δ _{N∞}	[mm/(N/mm²)]	-	-	0,90	0,78	-	-	

AT-HP / AT-HP PLUS / S&P ResAC-19 with rebar			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Temperature range I: 40°C / 24									
Displacement	δηο	[mm/(N/mm²)]	0,03	0,03	0,04	0,04	0,05	0,06	0,07
Displacement	δN∞	[mm/(N/mm²)]	0,07	0,09	0,10	0,12	0,13	0,17	0,20
Temperature range II: 80°C / 50°C									
Displacement	δ_{N0}	[mm/(N/mm²)]	0,04	0,04	0,05	0,06	0,07	0,08	0,10
Displacement	δn∞	[mm/(N/mm²)]	0,10	0,13	0,15	0,17	0,19	0,23	0,29

Calculation of displacement under service load: τ_{Sd} design value of bond stress Displacement under short term loading = $\delta_{N0} \cdot \tau_{Sd}/1,4$ Displacement under long term loading = $\delta_{N\infty} \cdot \tau_{Sd}/1,4$

Displacement under shear load 2)

AT-HP / AT-HP PLUS / S&P ResAC-19 with threaded rods			M8	M10	M12	M16	M20	M24
Displacement	δνο	[mm/kN]	0,06	0,06	0,05	0,04	0,04	0,03
Displacement	δν∞	[mm/kN]	0,09	0,08	0,08	0,06	0,06	0,05

AT-HP / AT-HP PLUS / S&P ResAC-19 with rebar			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Displacement	δνο	[mm/kN]	0,06	0,05	0,05	0,04	0,04	0,04	0,03
Displacement	δγ∞	[mm/kN]	0,09	0,08	0,07	0,06	0,06	0,05	0,05

Calculation of displacement under service load: V_{Sd} design value of shear load. Displacement under short term loading = $\delta_{N0} \cdot V_{Sd}/1,4$ Displacement under long term loading = $\delta_{V\infty} \cdot V_{Sd}/1,4$

AT-HP / AT-HP PLUS / S&P ResAC-19 Design Displacements for threaded rods and rebars Annex C9